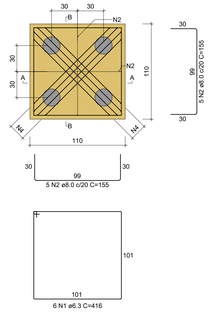
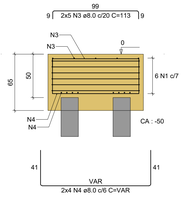
B1=B9
4xR20 maior
PLANTA
ESC 1:25

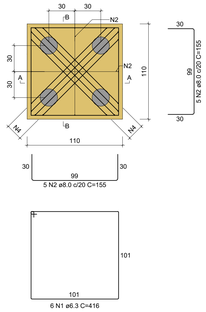


CORTE A-A = CORTE B-B
ESC 1:25

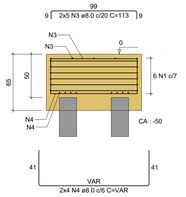


ARMADURA COM COMPRIMENTO VARIÁVEL				
N4	QUANT	C. VAR (cm)	C. UNIT (cm)	C. TOTAL (cm)
A	4	121	199	796
B	4	134	212	848

B2=B8
4xR20 maior
PLANTA
ESC 1:25



CORTE A-A = CORTE B-B
ESC 1:25



ARMADURA COM COMPRIMENTO VARIÁVEL				
N4	QUANT	C. VAR (cm)	C. UNIT (cm)	C. TOTAL (cm)
A	4	121	199	796
B	4	134	212	848

RELAÇÃO DO AÇO

2xB6 2xB9 3xB7 2xB8

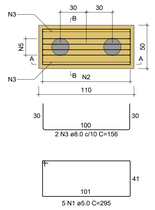
AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA50	1	6.3	54	416	22464
	2	8.0	90	155	13950
	3	8.0	90	113	10170
	4	8.0	72	VAR	VAR

RESUMO DO AÇO

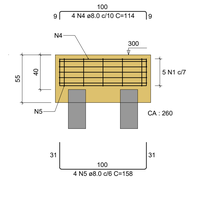
AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	6.3	224.6	60.5
	8.0	388.8	168.8
PESO TOTAL (kg)			
CA50	229.2		

Volume de concreto (C-25) = 6.91 m³
Área de forma = 25.74 m²

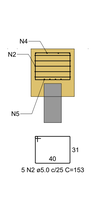
B10=B11=B12=B13
2xR20 maior
PLANTA
ESC 1:25



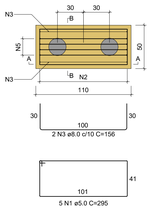
CORTE A-A
ESC 1:25



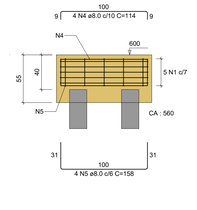
CORTE B-B
ESC 1:25



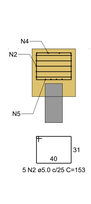
B14=B15=B16=B17=B18
2xR20 maior
PLANTA
ESC 1:25



CORTE A-A
ESC 1:25



CORTE B-B
ESC 1:25



RELAÇÃO DO AÇO

4xB13 5xB18

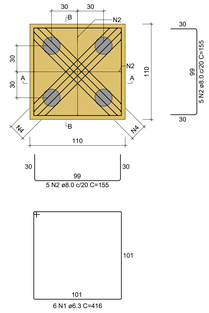
AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA60	1	5.0	45	295	13275
	2	5.0	45	153	6885
CA50	3	8.0	18	156	2808
	4	8.0	36	114	4104
	5	8.0	36	158	5688

RESUMO DO AÇO

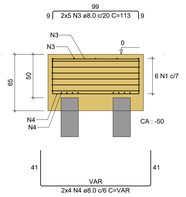
AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	8.0	126	54.7
CA60	5.0	201.6	34.2
PESO TOTAL (kg)			
CA50	54.7		
CA60	34.2		

Volume de concreto (C-25) = 2.64 m³
Área de forma = 15.84 m²

B3=B5=B7
4xR20 maior
PLANTA
ESC 1:25



CORTE A-A = CORTE B-B
ESC 1:25



ARMADURA COM COMPRIMENTO VARIÁVEL				
N4	QUANT	C. VAR (cm)	C. UNIT (cm)	C. TOTAL (cm)
A	4	121	199	796
B	4	134	212	848

RELAÇÃO DO AÇO

9xP1

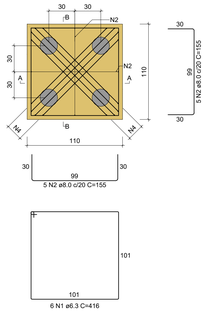
AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA60	1	5.0	36	29	1044
CA50	2	5.0	36	147	5292
	3	12.5	54	110	5940

RESUMO DO AÇO

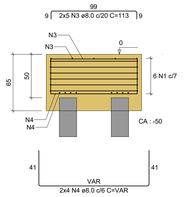
AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	12.5	59.4	62.9
CA60	5.0	63.4	10.7
PESO TOTAL (kg)			
CA50	62.9		
CA60	10.7		

Volume de concreto (C-25) = 0.00 m³
Área de forma = 0.00 m²

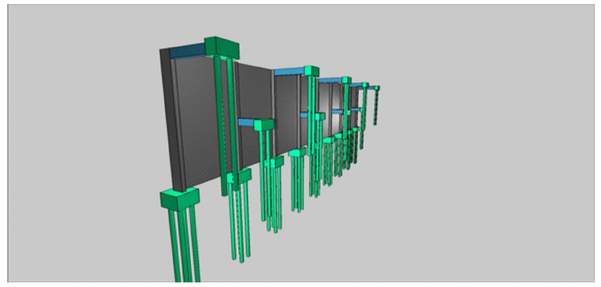
B4=B6
4xR20 maior
PLANTA
ESC 1:25



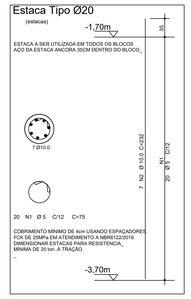
CORTE A-A = CORTE B-B
ESC 1:25

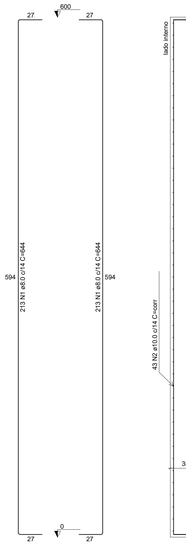


ARMADURA COM COMPRIMENTO VARIÁVEL				
N4	QUANT	C. VAR (cm)	C. UNIT (cm)	C. TOTAL (cm)
A	4	121	199	796
B	4	134	212	848



Estaca Tipo Ø20
(estacas)





RELAÇÃO DO AÇO

PAR1

AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA50	1	8.0	426	644	274344
	2	10.0	86	corr	256280

RESUMO DO AÇO

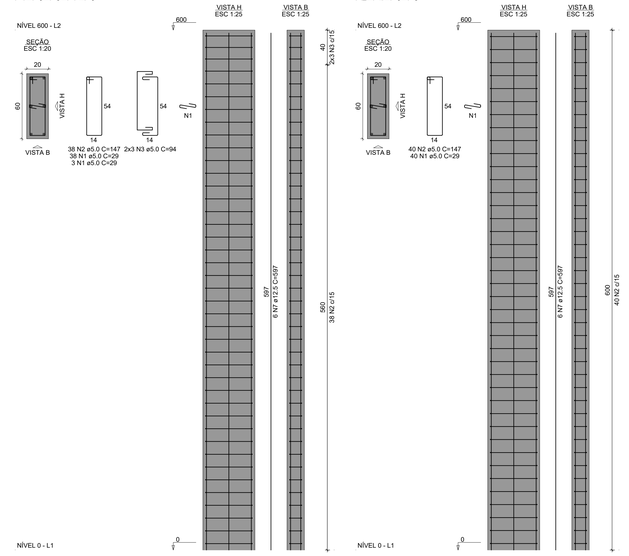
AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	8.0	2743.4	1190.8
	10.0	2562.8	1738.1
PESO TOTAL (kg)			
CA50	2928.8		

Volume de concreto (C-25) = 62.58 m³
Área de forma = 368.03 m²

PAR1

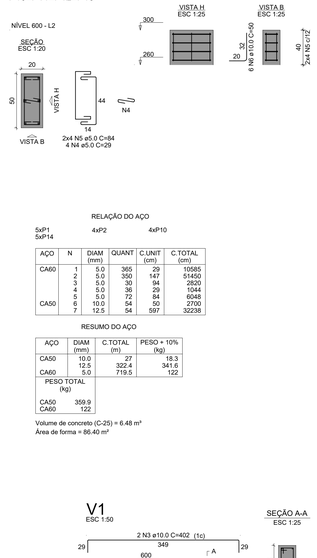
ESC 1:25

P1=P3=P5=P7=P9



P2=P4=P6=P8

P10=P11=P12=P13



RELAÇÃO DO AÇO

5xP1 5xP14 4xP2 4xP10

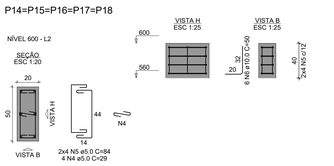
AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA60	1	5.0	365	29	10585
	2	5.0	350	147	51450
	3	5.0	30	94	2820
	4	5.0	36	29	1044
	5	5.0	72	84	6048
CA50	6	10.0	54	50	2700
	7	12.5	54	597	32238

RESUMO DO AÇO

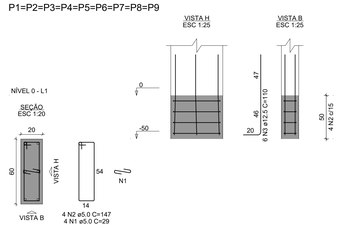
AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	10.0	27	18.3
	12.5	322.4	341.6
CA60	5.0	719.5	122
PESO TOTAL (kg)			
CA50	359.9		
CA60	122		

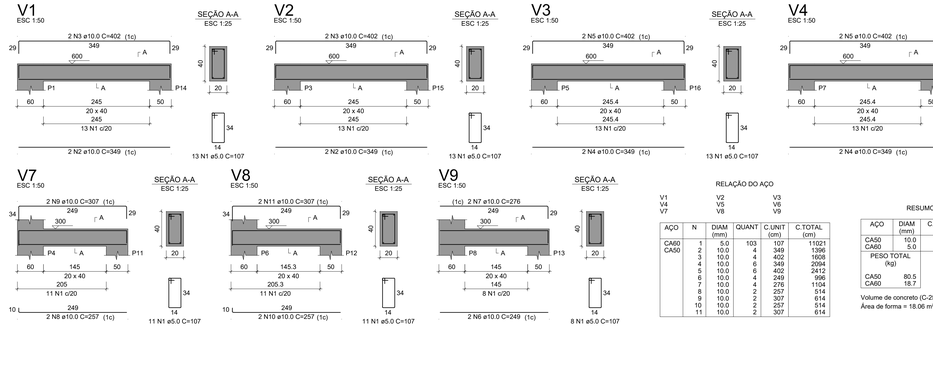
Volume de concreto (C-25) = 6.48 m³
Área de forma = 86.40 m²

P14=P15=P16=P17=P18



P1=P2=P3=P4=P5=P6=P7=P8=P9





V1
ESC 1:50

SEÇÃO A-A
ESC 1:25

V2
ESC 1:50

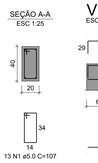
SEÇÃO A-A
ESC 1:25

V3
ESC 1:50

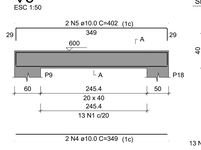
SEÇÃO A-A
ESC 1:25

V4
ESC 1:50

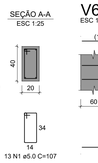
SEÇÃO A-A
ESC 1:25



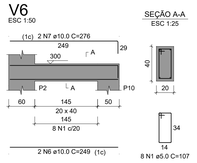
V5
ESC 1:50



SEÇÃO A-A
ESC 1:25



V6
ESC 1:50



SEÇÃO A-A
ESC 1:25

V7
ESC 1:50

SEÇÃO A-A
ESC 1:25

V8
ESC 1:50

SEÇÃO A-A
ESC 1:25

V9
ESC 1:50

SEÇÃO A-A
ESC 1:25

RELAÇÃO DO AÇO

V1 V2 V3
V4 V5 V6
V7 V8 V9

AÇO	N	DIAM (mm)	QUANT	C.UNIT (cm)	C.TOTAL (cm)
CA60	1	5.0	103	107	11021
CA50	2	10.0	4	349	1396
	3	10.0	4	402	1608
	4	10.0	6	349	2094
	5	10.0	6	402	2412
	6	10.0	4	249	996
	7	10.0	4	276	1104
	8	10.0	2	257	514
	9	10.0	2	307	614
	10	10.0	2	257	514
	11	10.0	2	307	614

RESUMO DO AÇO

AÇO	DIAM (mm)	C.TOTAL (m)	PESO + 10% (kg)
CA50	10.0	118.7	80.5
CA60	5.0	110.2	18.7
PESO TOTAL (kg)			
CA50	80.5		
CA60	18.7		

Volume de concreto (C-25) = 1.45 m³
Área de forma = 18.06 m²

CONTENÇÕES DO PAVIMENTO NÍVEL 0

Escala 1:50

CONTENÇÕES DO PAVIMENTO NÍVEL 600

Escala 1:50

	MRB Construtora Ltda Rua Pedro Ferreira Borges, 22 - Centro - Dona Euzebia - MG/Cel. (32)99918-9344 / engenheiroeduardoborges@gmail.com
	Projeto Estrutural
ENDEREÇO/OBRA:	Muro de Contenção entre as Ruas São Francisco de Assis e Padre Baião, Centro, Guidoval - MG
CONTEÚDO:	Muro de Arrimo
PROPRIETÁRIO:	Prefeitura Municipal de Guidoval
RESPONSÁVEL TÉCNICO:	Eduardo Nascimento Borges
CREA:	MG-239.188/D
ESCALA: INDICADA	**DATA: 06/26** **PRANCHA 2/2**